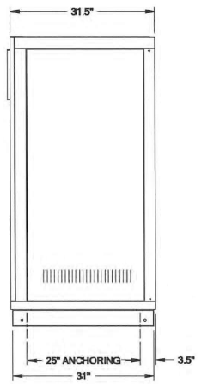
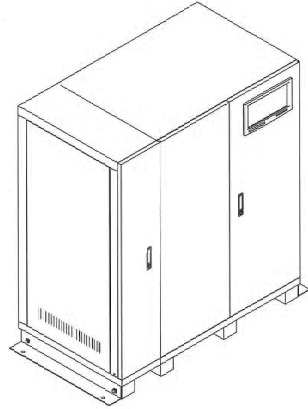


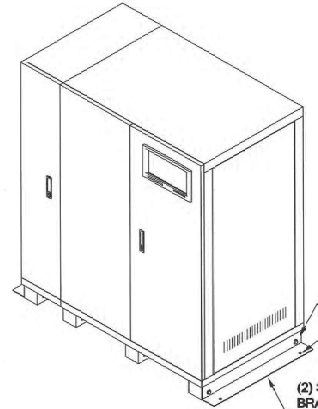
FRONT



SIDE



ISO FRONT LEFT

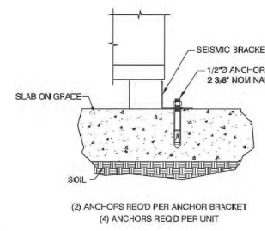


ISO FRONT RIGHT

(80KVA THRU 160 KVA)

- NOTES:**
- 1) ASSEMBLE SEISMIC BRACKETS TO CABINET USING SUPPLIED 1/2-13 HEX BOLT, PRIOR TO SECURING CABINET TO FINAL LOCATION.
 - 2) INDICATED MOUNTING HARDWARE TO BE SUPPLIED BY CUSTOMER, REFER TO LOCAL CODES FOR 30.3" TYPE AND LENGTH.

SIZE	DWG	REV
B	6001-277	B
DATE:	DRG. NO.	SHEET 2 OF 2



1 ANCHOR DETAIL

- NOTES:**
1. DESIGNED PER THE 2018 IBC / 2019 CBC / 2020 ABC, $F_a = 1.0$ & $S_s = 2.2$
 2. STORAGE CAPACITY: 4,000# MAX. WEIGHT.
 3. ANCHORS: HILTI KWIK-BOLT T2, 1/2" x 8.5" 1917W LABC SUPPLEMENT
 4. CONCRETE: 5" THICK x 2,500 PSI.
 5. SOIL BEARING PRESSURE: 500 PSF (MIN. REQ'D)
 6. EVALUATION BASED ON NORTH-FACE LOCATION (ONE OF THE HIGHEST LA FAL-T' AREAS) WITH THE FOLLOWING CALCULATION AS A TYPICAL EXAMPLE (ASSUMED GROUND FLOOR INSTALLATION)

POWER COMPANY
NORTHridge, CA 91324

LOADS & DISTRIBUTION: INVERTER CABINET
ANALYSIS BASED ON SECTION 13.5 OF THE ASCE 7-16 SPECIFICATION REFERENCED IN CHAPTER 16 OF THE 2018 IBC/2015 CBC/2020 ABC

F_p (13.3-1) = $0.4 \times a_p \times S_{ps} \times W_p / (R_p/I_p)$	0.234 x W_p	
F_p (13.3-2) = $1.6 \times S_{ps} \times I_p \times W_p$	2.336 x W_p	SHALL NOT BE GREATER THAN
F_p (13.3-3) = $0.3 \times S_{ps} \times I_p \times W_p$	0.438 x W_p	SHALL NOT BE LESS THAN

SITE CLASS = D	
$F_a = 1.2$	
$S_s = 1.83$	
$S_{ps} = 1.46$	
$I_p = 1.00$	ASCE 7-16 Table 13.5-1
$R_p = 2.5$	ASCE 7-16 Table 13.5-1
$I_p = 1$	

$W_p = 4000$ LB
 $0.7F_p = 0.7 \times 0.438 \times W_p = 0.31 \times 4000$ LB = 1,226 LB

OVERTURNING ANALYSIS:

CABINET HEIGHT, $H_c = 63.0$ IN
ANCHORS SPACING, $D = 25.0$ IN

$M_{ot} = V_{total} \times (H_c/4)$
= 1226 LB \times 63 IN \times $1/2$
= $36,632$ IN-LB

$M_{se} = W_p \times D/2$
= 4000 LB \times 25 IN/2
= $50,000$ IN-LB

$P_u \times H_c = (M_{ot} - 0.6 \times M_{se})/C$
= $(36632$ IN-LB $- 0.6 \times 50000$ IN-LB)/25 IN
= 345 LB <= UPLIFT

ANCHORS

ALLOWABLE CAPACITY PER ICC REPORT AND ACI 318-14 CHAPTER 17
PULLOUT: 830 LB
SHEAR: 900 LB

COMBINED STRESS = $(845$ LB/1650 LB) + $(1226$ LB/3600 LB)
= 0.55
< 1.2 O.K.

USE 1/2" ϕ x 2-3/8" MIN. EMBEDED, HILTI KB-T2 (ICC ESR-1917) OR APPROVED EQUAL.
(4) PER CABINET

CALCULATIONS

REVISION	DATE	BY	DESCRIPTION

EST. 1985
SEIZMIC ENGINEERING, INC.
133 E. Cypress St.
Covina, CA 91724
Tel: 626.966.9993

DESIGNED BY: M.V. / J.C.
DATE: 03/13/2023
LAST REV. BY:
REV. DATE:
TYPE:
SCALE: N.T.S.
APP'D BY: SALE / NREB



DESCRIPTION:
CABINET DETAILS

DRAWING NUMBER:
20-0491-D

JAN 28 2023